

APPLICATION FOR LAND SUBDIVISION (PLAT)

DATE RECEIVED: 5/6/2021
CHECK ONE: Preliminary Plat Final Plat Replat Amended Cancellation

1. PROPOSED SUBDIVISION NAME: Beckham Estates UNIT NO. _____
LOCATION DESCRIPTION/NEAREST COUNTY ROAD: 4600
ACREAGE 11.8 NO. OF LOTS: EXISTING 10 PROPOSED 10 11
REASON(S) FOR PLATTING/REPLATTING: New Subdivision

2. OWNER/APPLICANT*: Joe P. Jennings
(*If applicant is person other than owner, a letter of authorization must be provided from owner)
ADDRESS: 2135 County Road 4500 Sulphur Springs, TX 75482
TELEPHONE: 903 434 2509 FAX: _____ MOBILE: _____
EMAIL: _____

3. LICENSED ENGINEER/SURVEYOR: Est / Dynamic Engineering
MAILING ADDRESS: 101 Bill Bradford Suite 13 Sulphur Springs TX
TELEPHONE: 903 438 2422 FAX: 903 438 9955 MOBILE: _____
EMAIL ADDRESS: Stephen H@estinc.com

4. LIST ANY VARIANCES REQUESTED: _____
REASON FOR REQUEST (LIST ANY HARDSHIPS): _____
5. PRESENT USE OF THE PROPERTY: Residential
INTENDED USE OF LOTS: (CHECK ALL THOSE THAT APPLY)
 RESIDENTIAL (SINGLE FAMILY) _____ RESIDENTIAL (MULTI-FAMILY)
_____ OTHER (SPECIFY) _____

6. PROPERTY LOCATED WITHIN CITY ETJ: _____ YES _____ X NO
If yes, Name of City: _____

7. IS ANY PART OF THE PROPERTY IN A FLOODPLAIN? _____ YES _____ X NO

WATER SUPPLY: NHWS CORP ELECTRIC SERVICE: Oncore

SEWAGE DISPOSAL: Clear Water GAS SERVICE: Propane

8. Is the property subject to any liens, encumbrances, or judgments? If so, give details. (Provide separate sheet if needed) Permission from any lien holders and/or removal of any encumbrances or judgments will be necessary prior to filing of said plat with the County Clerk's Office.

9. See platting requirements. All necessary documents to reflect compliance must be complete before application will be deemed complete.

10. I agree to comply with all platting and subdivision requirements of Hopkins County, Texas. I understand that the plat will NOT be forwarded to the Commissioners' Court unless all documentation is satisfactorily filed with the County Clerk's Office correction due date.

Joe Jennings
Signature of Owner/Applicant

Joe Jennings Owner
Print Name & Title

**If applicant is person other than owner, a letter of authorization must be provided from owner. Signature indicates authorization for plat application and acceptance of waiver statement.

DATE: 5-6-2021

Appendix C
SUBDIVISION PLATTING CHECKLIST
SECOND (FINAL) READING

Subdivision name: Beckham Addition

YES	NO	N/A	
✓	—	—	All information required for preliminary plat.
✓	—	—	Lot and block numbers.
✓	—	—	Street names, <i>must be pre-approved by 9-1-1 Coordinator.</i>
✓	—	—	Acreage of each lot or parcel.
✓	—	—	Name and address of Surveyor/Engineer.
✓	—	—	Location and size of drainage structures.
✓	—	—	Location, size, and proposed use of easements.
—	—	X	Incorporated City's Boundary/ETJ Note.
✓	—	—	Servicing Utilities Note.
✓	—	—	✓ Certification from licensed professional engineer regarding utilities.
—	—	—	Restrictive covenants.
—	—	—	✓ Tax certificates and rollback receipts if required.
—	—	—	Home Owners' Association Incorporation articles and by-laws.
✓	—	—	↓ Construction plans of roads and drainage improvements.
✓	—	—	↓ Receipt showing payment of Final plat fees.
—	—	X	Sign-off for TxDOT road access, if applicable.
—	—	—	Appendix D (1) – Certificate of Dedication by Owner (when owner is an individual)
—	—	—	Appendix D (2) – Certificate of Dedication by Owner (when owner is a corporation)

FINAL CHECKLIST

YES NO N/A

___	___	___	Appendix D – Certificate of Recording (if applicable)
___	___	___	Appendix E – Water Supply Certificate
___	___	___	Appendix F – Certificate of Surveyor
___	___	___	Appendix G – Certificate of Engineer
___	___	___	Appendix H – Certificate of Road Maintenance (when roads are to be retained as private roads)
___	___	___	Appendix I – Certificate of County Approval (not applicable until the Court hears request to assume maintenance of roads)
___	___	___	Appendix J – Hopkins County Permit to Construct Access Driveway Facilities on County Road Right-of-Way
___	___	___	Appendix K – Lienholder’s Acknowledgement
___	___	___	Appendix L – Revision to Plat
___	___	___	Appendix O - On-Site Sewage Facility Inspector’s Approval
___	___	___	Appendix P - Utility Line Installation Permit
___	___	___	Appendix Q - Road Construction Specifications (Typical Section)
___	___	___	Appendix R - Cattle guard specification

Signature of Reviewer

Date of Review

ADDITIONAL REQUIREMENTS:

ALL ITEMS ON THIS CHECKLIST MUST BE IN THE HANDS OF THE COUNTY CLERK’S OFFICE NO LESS THAN THIRTY (30) DAYS PRIOR TO THE COMMISSIONERS’ COURT HEARING DATE.

DATE 05/06/2021

HOPKINS COUNTY CLERK
128 JEFFERSON STREET, SUITE C
SULPHUR SPRINGS TEXAS 75482

RECEIPT # 207803

TIME 11:18

FILE # M29816

RECEIVED OF: JOE JENNINGS

FOR: BECKHAM ADDITION

DESCRIPTION: APPLICATION FEES PAID / PRELIMINARY W/10 LOTS &
FINAL PLAT/TS/TS

AMOUNT PAID -----
\$1,450.00

* DUPLICATE RECEIPT *

PAYMENT TYPE K
CHECK NO 14664
COLLECTED BY TS

TAX CERTIFICATE

ACCT # 65-0351-000-004-02
 DATE 05/06/2021
 CC



HOPKINS COUNTY TAX OFFICE
 PO BOX 481
 SULPHUR SPRINGS, TX 75483
 (903) 438-4063

Cert# 201636
 FEE 10.00

Property Description			
ABS: 351, TR: 4-02, SUR: GANT JOHN J			PROP TYPE-D1 PCT OWNER-100.000
TOWN -		LOCATION-	N SH 19
ACRES - 35.700			

Values			
LAND MKT VALUE	74,970	IMPR/PERS MKT VAL	
LAND AGR VALUE	5,280	MKT. BEFORE EXEMP	5,280
EXEMPTIONS GRANTED:	NONE	LIMITED TXBL. VAL	

JENNINGS JOE & PATRICIA
 2135 CR 4586

SULPHUR SPRINGS TX 75482-0835

hereby certify and otherwise guarantee that the tax levies, penalties, and attorney fees due in the current month for the above described property are as listed below.

TAXES 2020	LEVY	P&I	ATTY FEES	AMT DUE
	.00	.00	.00	.00
	-----	-----	-----	-----
	.00	.00	.00	.00
				=====
		TOTAL DUE 05/2021		.00
		TOTAL DUE 06/2021		.00

ACCT # 65-0351-000-004-02

BREAKDOWN OF TAX DUE BY JURISDICTION

JURISDICTION	LEVY	P&I	ATT FEES	TOTAL
COUNTY	.00	.00	.00	.00
HOSPITAL	.00	.00	.00	.00

(CERTIFICATE MAY NOT INCLUDE ALL TAXING JURISDICTIONS)

TAX LEVY FOR THE CURRENT ROLL YEAR: COUN	32.99
TAX LEVY FOR THE CURRENT ROLL YEAR: HOSP	13.20
TOTAL TAX LEVY FOR THE CURRENT ROLL YEAR	46.19

 * SUBJECT TO ROLL BACK *
 * SUBJECT TO ROLL BACK *

REQUESTED BY:
 JOE JENNINGS

Debra Mitchell

Signature of authorized officer of collecting office

NORTH HOPKINS WATER SUPPLY CORPORATION
9364 TEXAS HIGHWAY 19 N
SULPHUR SPRINGS, TX. 75482-1120

April 27, 2021

To whom it may concern:

North Hopkins Water Supply Corporation will supply water on County Road 4760 for Joe Jennings proposed sub-division. He will pay for the upgrade necessary to supply this water, according to our Engineer's specifications.

Edgar Clements, Jr. Manager
903-945-2619

A handwritten signature in black ink, appearing to read "Edgar Clements, Jr.", with a long horizontal flourish extending to the right.



5 May 2021

Oncor Electric Delivery
111 Heritage Ct.
Sulphur Springs, TX 75482

Re: New Subdivision at CR 4760 & PR 4860 - Sulphur Springs, TX 75482

Please be advised that Oncor Electric Delivery Company LLC, a Delaware limited liability company, can provide electric service to the above referenced site. Service will be provided upon request in accordance with our tariffs and service regulations on file with the Public Utility Commission of Texas.

If you have questions or need additional information, please feel free to contact me.

Sincerely,

Ryan Young
Utility Designer

Appendix O

CERTIFICATE OF ON-SITE SEWAGE FACILITY INSPECTOR'S APPROVAL

THE STATE OF TEXAS §

COUNTY OF HOPKINS §

KNOW ALL MEN BY THESE PRESENTS, that I, the undersigned, a Licensed On-Site Sewage Facility Inspector in the State of Texas, hereby certify that I have inspected the On-Site Sewage Facilities for this plat, and the same complies with the related requirements of the Hopkins County Subdivision Regulations and the TCEQ.

Kristy Springfield
On Site Inspector

May 6, 2021
Date

License No. OS 0034831

Seal:



[NOTE: *The inspector may be required to be present for questioning at the presentation of the plat to the Commissioners' Court.*]

APPLICATION FOR VARIANCE

Now comes Joe Jennings, Applicant, who requests permission from the Hopkins County Commissioners Court for a variance from the County Subdivision Rules and Regulations.

Applicant makes this Variance Application to resolve practical difficulties or unnecessary physical hardships that have resulted from the size, shape, dimensions, or other physical conditions of the location or in the immediate vicinity of the property described in the attached exhibit.

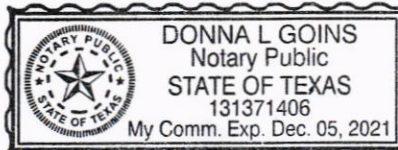
Specifically, Applicant requests the Variance for the following reasons:

A variance to the area regulation requirement of a minimum of one acre, free of easements; to allow for a request from ONCOR to place a ten foot easement across the front of the lots in the Beckham Addition for electric service. The variance, if granted, will require all OSSF spray heads to be located on the rear one half acre of each lot.

Joe Jennings
Signature of Applicant

Signed and sworn before Donna L. Goins, Notary Public, on the 9th day of October, 2020.

Donna L. Goins
Notary Public



ORDER

The Hopkins County Commissioners Court, having reviewed the Application for Variance filed by

_____ hereby grants denies (check decision of the Court) the Application.

Filed this the 12 day of October, 2020.

Robert M. Jensen
County Judge

Untitled Map

Write a description for your map.

Legend



BOUNDARY

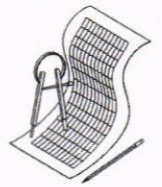


DEEDSKETCH





GEOTECHNICAL INVESTIGATION



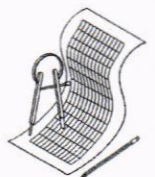
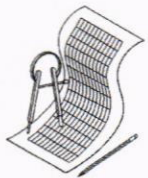
GEO-TECHNICAL INVESTIGATION
PROPOSED PAVING
FOR
JOE JENNINGS
COUNTY ROAD 4760
SULPHUR SPRINGS TEXAS
PROJECT # 7679

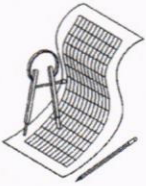
SUBMITTED BY:

DYNAMIC
Engineering
Consultants
PLLC

REPORT NUMBER:
DYNGEO-7679

August 18, 2020





DYNAMIC Engineering Consultants PLLC



From: J.W. Burnett
Dynamic Engineering
200 South Hillcrest Drive
Sulphur Springs, Texas 75482
903-513-3773

Date: August 18, 2020

To: Mr. Joe Jennings
County Road 4760 Sulphur Springs, Texas

**Project
Number:** DYNNGEO-7679

Subject: Geotechnical Investigation

Purpose:

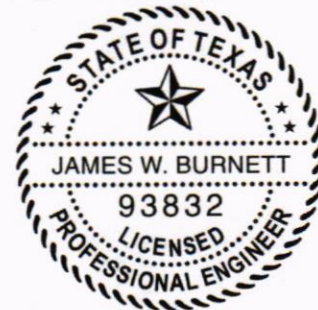
The Purpose of this report is to present the results and recommendations of the geotechnical investigation performed for Mr. Joe Jennings sub-division located at County Road 4760 in Sulphur Springs Texas. This report includes the following information and recommendations:

- Field Observation and Boring Locations
- Soil Parameters Necessary for Paving Design Based Upon:
 - Visual Inspection and Onsite Testing
 - Laboratory Testing
- Site Preparation
- Pavement Design Recommendations

If additional services are needed, such as or construction materials testing, please contact us at the number listed above. Thank you for confiding in Dynamic Engineering for your consulting needs.

Sincerely,

James W. Burnett, PE
Project Engineer
Dynamic Engineering



Firm Registration # F-8215

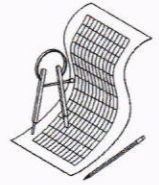
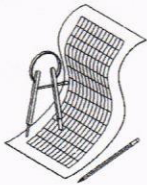


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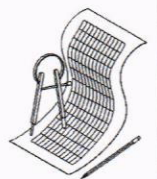
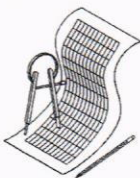
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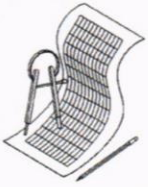
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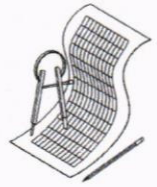
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DYNAMIC *Engineering* *Consultants* PLLC



Introduction:

The purpose of this study was to explore the subsurface conditions at the site to enable an evaluation of an acceptable paving design for the proposed construction. Our scope of services included collecting continuous soil samples at three (3) locations inside the proposed paving footprint. The depth of the bore holes were approximately 4 feet. Select laboratory testing and preparation of this geotechnical report are also included in the scope of effort. This report briefly outlines the testing procedures, presents available project information, describes the site and subsurface conditions, and presents recommendations regarding the following:

- Grading procedures for site development
- Paving Design
- Comments regarding factors that will impact construction and performance of the proposed construction.

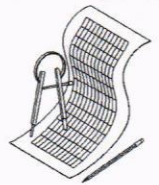
Project Description:

An agricultural tract is currently under construction to be a rural sub-division in Hopkins County. The construction consists of a sub-division access road that ends in a cul-de-sac located in near Sulphur Springs Texas.

Core samples were collected from three boreholes to a depth of 4' inside the footprint of the proposed paving. Authorization to conduct the geo-technical investigation was granted by Mr. Joe Jennings. Sampling and Field-testing was started and completed on Tuesday June 30th 2020.



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Reference Figure 1 for a picture of the Geo Probe 540 MT during soil collection.



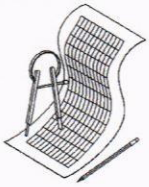
Figure 1: Picture of the Geo Probe During Soil Collection at Bore Hole 1.

Field Operations & Lab Testing:

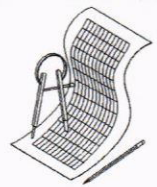
The site subsurface conditions were explored with three (3) soil sample sets taken inside the proposed footprint of the property. Boring depth was approximately 4 feet below ground surface. The boring locations are identified on the Bore Hole Location and Vicinity Map (Figure A1) in Appendix A.

The borings were advanced using a GeoProbe direct push/hydraulic hammer system. Sample collection and field tests were performed in general accordance with ASTM procedures or other accepted methods.

Undisturbed samples of soils were obtained using a Macro Core sampling tube. The Macro Core sampling tube extrudes the sample into a clear PVC liner. The Liner is 1.5" in diameter and 48" in length. One (1) sample of approximately 48" in length is collected for each bore hole. The samples are identified on the liner according to boring number and depth. Dynamic Cone Penetrometer tests are performed at the depth of the bore hole following the removal of each sample. After logging and visual inspection of the sample, it is sealed for transport to the lab.



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Selected soil samples were tested in the laboratory to determine material properties for our evaluation. These tests include: Moisture content, percent passing the #200 sieve (wet sieve), Atterberg limits, Hydrometer particle size analysis, Pocket Penotrometer, and unconfined compressive strength. The laboratory testing was performed in general accordance with the ASTM procedures. The bore logs are located in Appendix B.

Soil Characteristics:

The soils at the site consist of the following:

- The surface layer consists of approx. 4' of high to moderately expansive, black and brown, Fat Clay and Lean Clay soil. The Plasticity Index (PI) ranged from low 20's to low 40's. The USCS classifications are CH and CL. The soil is described as stiff. The allowable soil bearing capacity is approx. 1500 psf for this layer. The moisture content at the time of testing was moderate.

Groundwater Information:

Groundwater was not encountered during sample collection. If more detailed water level information is required, observation wells or piezometers could be installed at the site, and water levels could be monitored. It should be noted that groundwater level fluctuations may occur due to seasonal and climatic variations, alteration of drainage patterns, leaking utilities, land usage, and ground cover. We recommend that the contractor determine the actual groundwater levels at the site at the time of the construction activities.

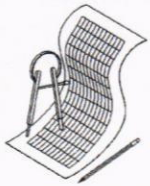
Pavement Recommendations:

Portland Cement Concrete Paving:

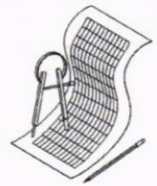
The specifications for construction of roads and streets are based on the requirement that an asphalt pavement or a concrete pavement will be constructed. The materials, design, specifications, and procedure shall conform to requirements as described in this document and the Hopkins county Sub-division Regulations.

Design of the concrete pavements should specify a minimum 28-day concrete compressive strength of 3,500 psi with 4 percent to 6 percent entrained air. Hand-placed concrete should have Min. slump of 4 inches and a max slump of 6". A sand-leveling course should not be permitted beneath pavements. The concrete should be placed within one and one-half hours of batching. During hot weather, the concrete placement should follow ACI 305 Hot Weather concreting guidelines. In no case should concrete temperature exceed 95 F. Consideration should be given to limiting concrete placement to the time of day, which will minimize large differences in the ambient and concrete temperature. Use of superplasticizer should be considered to improve the concrete workability without increasing water cement ratio.

Sealed contraction joints shall be installed at 3X the pavement section thickness in feet (i.e. 5" x 3 = 15' spacing). The saw cut depth shall be one-quarter of the pavement thickness. This spacing has historically exhibited less uncontrolled, post-construction cracking than pavements with wider joint spacings. As a minimum, isolation



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joints should be used wherever the pavement will abut a structural element subject to different movement levels, e.g., light poles, retaining walls, existing pavement, stairways, Entryway piers, building walls, or manholes. Contraction joints shall be installed at 90' max spacing. After construction, the isolation, construction, and contraction joints should be inspected periodically and resealed, as necessary. Reference Appendix O for joint details and specifications. The pavement should be nominally reinforced as follows:

- Light Duty: No. 3 bars, at 18 inches on center, each way.

Reference the table below for the PCCP section required for Sub-Division Development:

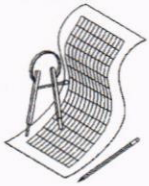
PCCP SECTION (CONCRETE PAVING)	
Layer Material	Thickness (in)
Portland Cement Concrete	5
Sub-Grade: Lime Stabilize @ 6% by wt. Per TxDOT Item 260 or Select Fill as a min. if PI>18 or Per Geo Report	6

Asphalt Hot Mix Base

The hot mix asphaltic concrete pavement coarse aggregate will be so crushed that a minimum of 80% of particles retained on #4 sieve will have more than one crushed face when tested in accordance with test method Tex.-413A (Particle Count). For Type "D" surface material, the asphaltic material will form from 5 to 8 percent of the mixture by weight. For Type "B" base material, the asphaltic material will form from 3.5 to 7 percent of the mixture by weight. For both surface and base material, the asphalt content used will be that percent required obtaining optimum density. This percent asphalt will be obtained from a mix design performed according to Texas Department of Transportation 1993 Standard Specifications. The mix design for base and surface material shall be reviewed and approved by the County Engineer. The actual asphaltic material contained in the delivered mix will be within a + 0.50% tolerance of the content specified in the mix design.

The HMAC will be installed at an application rate of 110/Lbs/SY/In of depth for both Type "D" and Type "B". At the seams where the new HMAC meets the existing HMAC, or concrete headers, or valley gutters, or curb and gutter, a tack coat (RC-250) will be applied to the seams at a rate of 0.05 Gal/SY.

When installing Type "D" HMAC on a Type "B" base, a tack coat (RC-250) will be applied on the base if the base if the base has been in place for more than three days, or if required by the Engineer. The tack coat will be applied at a rate not to exceed 0.50% Gal/SY and rolled with a pneumatic roller.



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The Type "D" and Type "B" HMAC will be installed with an approved HMAC laying machine, unless otherwise approved by the County Engineer. A motor grader is not approved to install HMAC.

Laydown operations will be conducted in such sequence that vehicles transporting asphaltic concrete material to this project will not travel over the completed pavement until said pavement will have been in place for a minimum of twenty-four hours, unless otherwise directed by the County Engineer. Joints will be staggered so that they fall at least 12" from the previous joint.

Storage of the completed mix upon the ground will not be permitted at the mixing plant or the job site. Any mix that comes into contact with earth or other objectionable foreign matter will be rejected.

Hot mix asphaltic concrete will be accepted for density and depth on a lot basis. A lot will consist of one day's production or 600 tons, whichever is less, and shall be divided into four equal sublots. One test shall be made for each subplot, unless the County Engineer judges the lot too small to warrant testing.

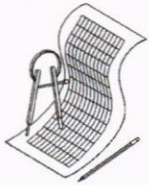
Each lot of pavement will be accepted, with respect to density, when the average field density is equal to or greater than 90.0 percent of the average maximum theoretical density as determined in accordance with ASTM D2041, and when no individual determination is less than 86.0 percent of the average maximum theoretical density. Four field density determinations will be made for each lot. Cores or sawed samples taken from the pavement will be used to determine the field density. The density of the cored or sawed samples shall be determined in accordance with ASTM D2726.

The specimen used to determine the average maximum theoretical density for a lot may be sampled by any one of the following four methods:

- (1) A sample may be removed from the truck delivering the HMAC for the lot being tested.
- (2) A sample may be removed from the HMAC laying machine placing the lot being tested.
- (3) A sample may be created by combining the material from the four individual core samples used for field densities.
- (4) A sample may be created from each individual core sample used for field densities, with the results being averaged.

Specimens used for field density determination shall be carefully crumbled, using heat if necessary. If heating is necessary, the specimen shall be heated to the lowest temperature required for proper preparation of the sample.

The use of nuclear field density determinations shall not be used as the basis for acceptance with respect to density.



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Each lot of pavement will be accepted, with respect to depth, when the average field depth deficiency is equal to or less than 0.25 inches for base courses and equal to or less than 0.13 inches for surface courses, and when no individual determination is deficient more than 1.00 inch for base courses and more than 0.50 inches for surface courses. Four field depth determinations will be made for each lot. Cores of sawed samples taken from the pavement will be used to determine the actual depth.

The surface course shall be a minimum of two inches (2") of HMAC meeting the specifications of Item 340, Type D, in the 1993 TX Dot Specifications.

Reference the table below for the (HMAC) section required for Sub-Division Development:

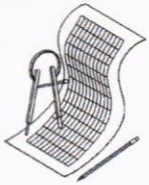
HMAC Section (Asphalt Paving)	
Layer Material	Thickness (in)
TxDot 340 Type D Surface Course	2
TxDOT 247 Type A or C Grade 2 Flex Base	6
Sub-Grade: Lime Stabilize @ 6% by wt. Per TxDOT Item 260 or Select Fill as a min. if PI>18 or Per Geo Report	6

All asphaltic mixtures shall be placed with an approved HMAC laying machine. The mix shall be compressed thoroughly and uniformly compacted immediately after placing to the required density. All compaction rolling shall be complete before the material cools below 175 degree F. The completed surface shall meet the approval of the County Engineer for riding surface, finish and appearance.

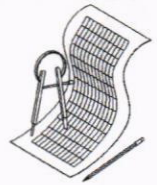
Subgrade Preparation:

The paving sub-grade shall be lime stabilized at a rate of 6% by dry weight to a depth of 6". The lime stabilization process shall conform to the specifications of TxDOT Item 260. In lieu of lime stabilization, 6" of properly compacted select fill can be installed below the paving section. Reference the "Select Fill" section of this document for material specifications and installation procedure.

The subgrade may be prepared and allowed to reach a Proctor Density of ninety-five percent (95%) at a min of -1 to +4% of the optimum moisture content through natural cycles of consolidation or may be rolled and watered where placement of the paving is to be done immediately. Testing shall be done at five hundred foot (500') intervals, with a minimum of two (2) tests, or wherever there is a change in the subgrade material. The subgrade must be inspected and approved by the Precinct Commissioner concerned or other person designated by the Commissioners' Court, in writing, prior to any application of base. Proctor Density test results must be presented



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to the Precinct Commissioner concerned or other designated person, and all preparatory work must be inspected and approved, in writing by the Precinct Commissioner or other designated person before any topping may be done.

Select Fill:

Select fill shall consist of homogeneous soils free of organic matter and rocks larger than four inches in diameter and possess an Atterberg plasticity index of 5 to 18, with a liquid limit of 35 or less. No more than 75% is allowed to pass the #200 sieve. The material should be placed in the following manner:

1. Prepare the subgrade in accordance with the recommendations discussed in a previous section of this report.
2. Place subsequent lifts of select fill in thin, loose layers not exceeding eight inches in thickness to the desired rough grade and compact to a minimum of 95 percent of the maximum density defined by ASTM D 698. Maintain moisture within -1% to +4% of theoretical optimum.
3. Conduct in-place field density tests at the following frequencies:
 - One test per 500 linear feet of road per lift or a minimum of 2 tests.
4. Prevent excessive loss of moisture during construction.

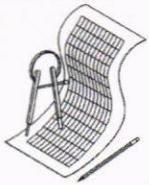
Note: Contact Dynamic Engineering Consultants to request a fee schedule for construction materials testing for this project.

ROAD SPECIFICATIONS

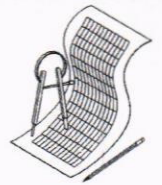
Rural Roadway Section:

(Minimum lot frontage, excluding cul-de-sacs, shall be 100 ft.)

Right-of-Way (minimum)	60'
Pavement Width	22'
Base Course Width (minimum)(if applicable)	24'
Subgrade Width	26'
Turnaround Right-of-Way	60' radius
Turnaround Pavement	41' radius



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Report Limitations:

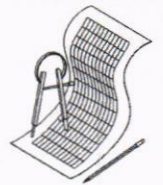
The recommendations submitted in this report, are based on the available subsurface information obtained by Dynamic Engineering Consultants and design details furnished by Joe Jennings for the proposed project. If there are any revisions to the plans for this project, or if deviations from the subsurface conditions noted in this report are encountered during construction, Dynamic Engineering Consultants should be notified immediately to determine if changes in the foundation recommendations are required. If Dynamic Engineering Consultants are not notified of such changes, we will not be responsible for the impact of those changes on the project.

The professional engineer warrants that the findings, recommendations, specifications, or professional advice contained herein have been made in accordance with generally accepted professional engineering practices in the local area. No other warranties are implied or expressed.

After the plans and specifications are more complete, the geotechnical engineer should be retained and provided the opportunity to review the final design plans and specifications to check that our engineering recommendations have been properly incorporated into the design documents. At this time, it may be necessary to submit supplementary recommendations. If Dynamic Engineering Consultants are not retained to perform these functions, we will not be responsible for the impact of those conditions on the project. This report has been prepared for the exclusive use of Mr. Joe Jennings.



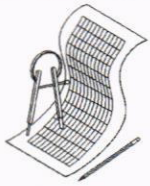
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Appendix A



Figure A 1: Bore Hole Locations and Vicinity Map



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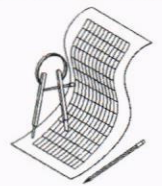


Figure A 2: Picture of Opened Soil Samples



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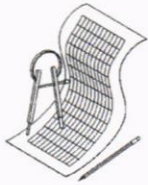
Unified Soil Classification System:

This soil system is based on the recognition of the type and predominance of the constituents, considering grain size, gradation, plasticity index, and liquid limit.

It contains three major divisions of soils:

- Coarse-grained
- Fine-grained
- Highly organic

The group symbols for each major soil division are located in Table A1. Some soils have characteristics of two groups because they are close to the borderline between the groups either in percentage of the various grain sizes or in plasticity characteristics. In cases like these, use the two group symbols, connected by a hyphen, which most nearly describe the soil. An example of this might be a SM-SC. This would be a sand, which has silt and clay binder. Those soils that are not readily identifiable in the field and the proper soil symbol designated necessitate sieve analysis and Atterberg limits tests. From these test results, the proper soil symbol can be determined.

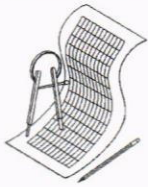


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Table A 1: Unified Soil Classification System

MAJOR DIVISIONS		GROUP SYMBOLS	DESCRIPTIONS
COARSE GRAINED SOILS More Than Half Retained on 200 Sieve	GRAVELS More Than Half Coarse Fraction Retained on No. 4 Sieve	Clean Gravels (Little or no Fines)	GW Well Graded Gravels, Gravel - Sand Mixtures, Little or no Fines
		Gravels With Fines (Appreciable Fines)	GP Poorly Graded Gravels, Gravel - Sand Mixtures, Little or no Fines
		GM	Silty Gravels, Gravel-Sand-Silt Mixtures
		GC	Clayey Gravels, Gravel-Sand-Clay Mixtures
	SANDS More Than Half Coarse Fraction Passes a No. 4 Sieve	Clean Sands (Little or no Fines)	SW Well Graded Sands, Gravelly Sands, Little or no Fines
		Sands With Fines (Appreciable Fines)	SP Poorly Graded Sands, Gravelly Sands, Little or no Fines
		SM	Silty Sands, Sand - Silt Mixtures
		SC	Clayey Sands, Sand - Clay Mixtures
FINE GRAINED SOILS More Than Half Passes 200 Sieve	SILTS and CLAYS Liquid Limit Less Than 50	ML	Inorganic Silts & Very Fine Sands, Silty or Clayey Fine Sands, Clayey Silts
		CL	Inorganic Clays of Low to Medium Plasticity, Lean Clays
		OL	Organic Silts & Organic Silty Clays of Low Plasticity
	SILTS and CLAYS Liquid Limit Greater Than 50	MH	Inorganic Silts, Fine Sand or Silty Soils, Elastic Silts
		CH	Inorganic Clays of High Plasticity, Fat Clays
		OH	Organic Clays of Medium to High Plasticity, Organic Silts
Highly Organic Soils		PT	Peat and Other Highly Organic Soils



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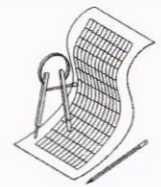


Table A2 shows the relationship between particle size and soil classification. Sieve and Hydrometer analysis can be used to determine the percentage of different particle sizes that exist in a sample.

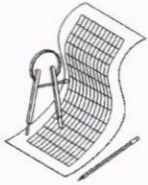
Table A 2: USCS Particle Size and Classification

Unified Soil Size Classification			
Millimeters	Inches	U.S. Standard Sieve Size	Particle Size
256 and above	12 and above		Boulder
72-256	3-12		Cobble
19-75	¾-3		Coarse Gravel
4.75-19	3/16-3/4	3/16"=4	Fine Gravel
2.4-4.75	3/32-3/16	3/32"=10	Coarse Sand
.42-2.4		.42mm=40	Medium Sand
.074-.42		.074mm=200	Fine Sand
.005-.074			Silt
.005 and below			Clay

Tables A3 and A4 use measurable attributes of soil such as Standard Penetration Test, Un-confined Compression Test results, and simple field tests to determine the descriptive terms for the soil sampled.

Table A 3: Terms for Soil Consistency

Soil Consistency Terms				
Coarse Grained Soils		Fine Grained Soils		
Descriptive Terms	No. Blows/ft (SPT)	Descriptive Terms	No. Blows/ft (SPT)	Unconfined Compression Tons/ft²
Very Loose	0-4	Very Soft	<2	<.25
Loose	4-10	Soft	2-4	.25-.50
Medium Dense	10-30	Medium Stiff	4-8	.50-1.0
Dense	30-50	Stiff	8-15	1.0-2.0
Very Dense	>50	Very Stiff	15-30	2.0-4.0
		Hard	>30	>4



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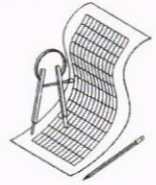


Table A 4: Thumb Penetration Resistance for Determining Soil Consistency Terms.

Penetration Resistance and Unconfined Compression Strength		
Consistency	Field Identification	Unconfined Compressive Strength tons/ft²
Very soft	Easily penetrated several inches by fist	Less than 0.25
Soft	Easily penetrated several inches by thumb	0.25–0.5
Medium	Can be penetrated several inches by thumb with moderate effort	0.5–1.0
Stiff	Readily indented by thumb, but penetrated only with great effort	1.0–2.0
Very stiff	Readily indented by thumbnail	2.0–4.0
Hard	Indented with difficulty by thumbnail	over 4.0



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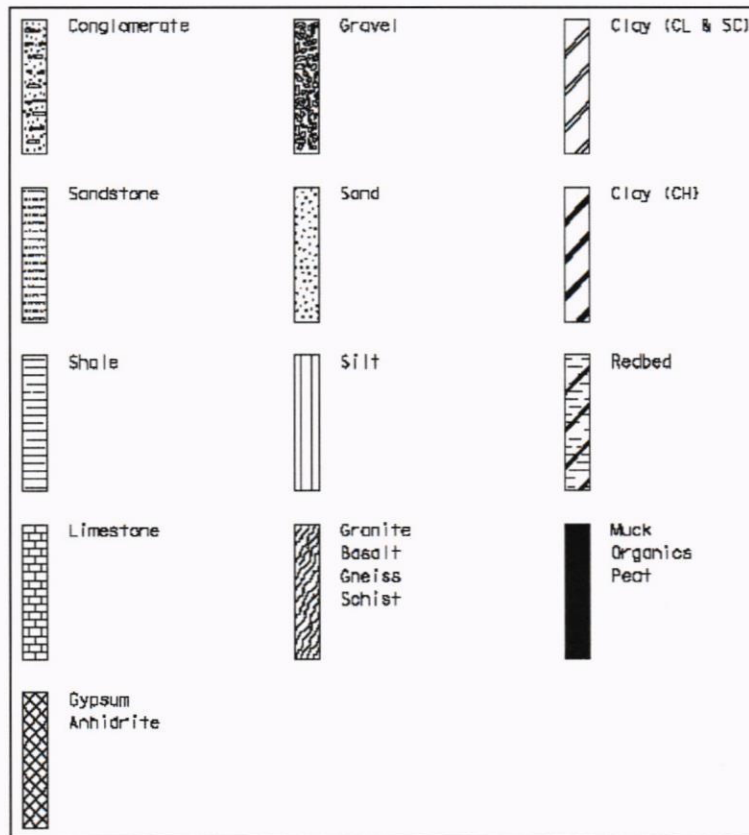
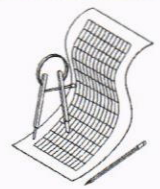


Figure A 3: Symbol for USCS Soil Classifications.

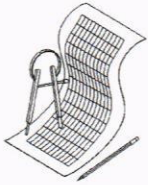


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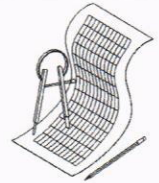


Table A 5: Terms Characterizing Soil Structure.

Slickensided	Having inclined planes of weakness that are slick and glossy in appearance.
Fissured	Containing shrinkage cracks, frequently filled with fine sand or silt. Usually vertical.
Laminated	Composed of thin layers of varying color and texture, usually grading from sand or silt at the bottom to clay at the top.
Crumbly	Cohesive soils which break into small crumbs upon drying.
Calcareous	Containing appreciable quantities of calcium carbonate. Usually nodular.
Well Graded	Having wide range in grain sizes and substantial amounts of all intermediate particle sizes.
Poorly Graded	Predominantly of one grain size (uniformly graded) or having a range of sizes with some intermediate size missing (gap or skip graded).

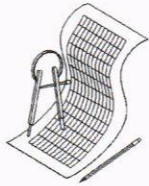


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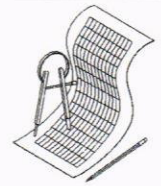


Appendix B

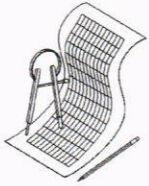
PROJECT # 7679			 DYNAMIC <i>Engineering</i> Consultants <i>PLLC</i>	DATE: 7/30/2020	SOIL BEARING (LB/SQ FT)	MOISTURE CONTENT (%)	ATTENBURG LIMITS (%)					
SAMPLES	USC	WATER LEVEL		PROJECT: JENNINGS			FIELD STRENGTH DATA	LIQUID LIMIT LL	PLASTIC LIMIT PL	PLASTICITY INDEX PI	% MINUS #200	% MINUS .002 MM (FINE CLAY)
				LOG # B1								
1	CH		FAT CLAY, black & brown, hard	P= 4.5	9000	18	52	18	34	91	22	
	CH		FAT CLAY, brown, hard	P= 4.5	9000	17	52	15	37	92	33	
5												
10												
15												
20												
WATER LEVEL: NONE ESTIMATED <input type="checkbox"/> MEASURED <input checked="" type="checkbox"/> PERCHED <input type="checkbox"/>			KEY TO ABBREVIATIONS N=SPT DATA (BLOWS/FT) P=POCKET PENETROMETER (TSF) D=DYNAMIC PENETROMETER (BLOWS/1.75') T=TORVANE (TSF)			NOTES: Boring terminated @ 4'						



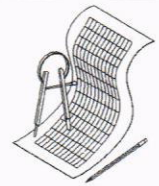
DYNAMIC Engineering Consultants PLLC



PROJECT # 7679				DATE: 7/30/2020	SOIL BEARING (LB/SQ FT)	MOISTURE CONTENT (%)	ATTENBURG LIMITS (%)			% MINUS #200	% MINUS .002 MM (FINE CLAY)
SAMPLES	USC	WATER LEVEL		PROJECT:			FIELD STRENGTH DATA				
				JENNINGS							
				LOG # B2							
				LL	PL	PI					
1	CH		FAT CLAY, black, very stiff	P= 3	6000	21	61	18	43	85	22
	CL		LEAN CLAY, brown, very stiff	P= 2.5	5000	16	40	21	19	77	23
5											
10											
15											
20											
WATER LEVEL: NONE ESTIMATED <input checked="" type="checkbox"/> MEASURED <input type="checkbox"/> PERCHED <input type="checkbox"/>			KEY TO ABBREVIATIONS N=SPT DATA (BLOWS/FT) P=POCKET PENETROMETER (TSF) D=DYNAMIC PENOTROMETER (BLOWS/1.75') T=TORVANE (TSF)			NOTES: Boring terminated @ 4'					



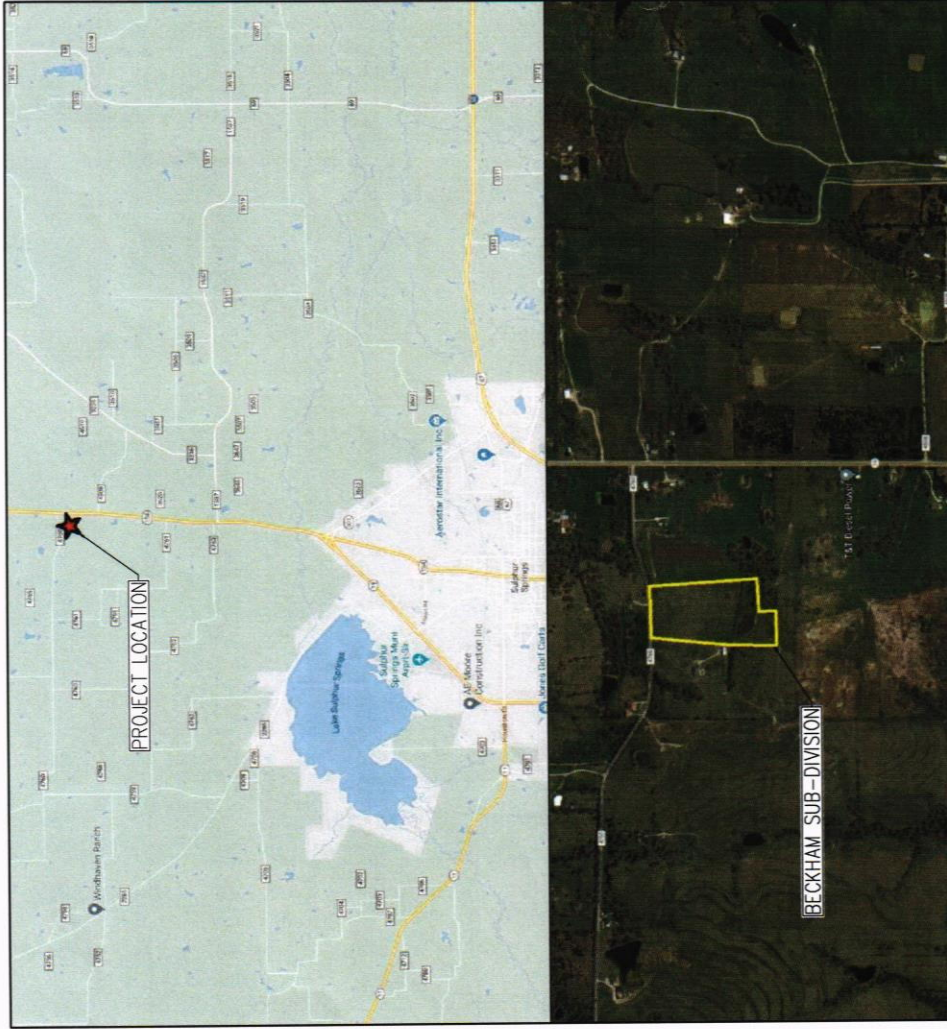
DYNAMIC Engineering Consultants PLLC



PROJECT # 7679			 DYNAMIC <i>Engineering</i> Consultants <i>PLLC</i>	DATE: 7/30/2020	SOIL BEARING (LB/SQ FT)	MOISTURE CONTENT (%)	ATTENBURG LIMITS (%)			% MINUS #200	% MINUS .002 MM (FINE CLAY)		
SAMPLES	USC	WATER LEVEL		PROJECT:			FIELD STRENGTH DATA	LIQUID LIMIT	PLASTIC LIMIT			PLASTICITY INDEX	
				JENNINGS									LOG # B3
1		CH	FAT CLAY, black, hard	P= 4	8000	20	56	28	28	94	26		
		CH	FAT CLAY, brown, very stiff	P= 2.75	5500	26	52	15	36	91	33		
5													
10													
15													
20													
WATER LEVEL: NONE			KEY TO ABBREVIATIONS			NOTES: Boring terminated @ 4'							
ESTIMATED			N=SPT DATA (BLOWS/FT)										
MEASURED			P=POCKET PENETROMETER (TSF)										
PERCHED			D=DYNAMIC PENETROMETER (BLOWS/1.75')										
			T=TORVANE (TSF)										

BECKHAM SUB-DIVISION

Sulphur Springs, Texas



ENGINEER:

DYNAMIC ENGINEERING
CONSULTANTS, PLLC
200 SOUTH HILLCREST DRIVE SUITE C
SULPHUR SPRINGS, TX 75482
CONTACT: JAMES W. BURNETT
PH# 903-513-3773

SHEET INDEX	
SHEET NO.	DESCRIPTION
T1.0	TITLE PAGE
C1.0	SITE PLAN
C1.1	PAVING DETAILS
C2.0	EROSION CONTROL NOTES
C2.1	EROSION CONTROL PLAN
C2.2	EROSION CONTROL DETAILS
C3.0	GRADING PLAN
C3.1	PROPOSED ROAD PLAN & PROFILE
C3.2	PRE-CONSTRUCTION DRAINAGE PLAN
C3.3	POST-CONSTRUCTION DRAINAGE PLAN
C3.4	PRE & POST DEVELOPMENT CALCULATIONS

BECKHAM SUB-DIVISION
SULPHUR SPRINGS, TX

PERMIT: 11042020
ISSUED FOR: DATE:

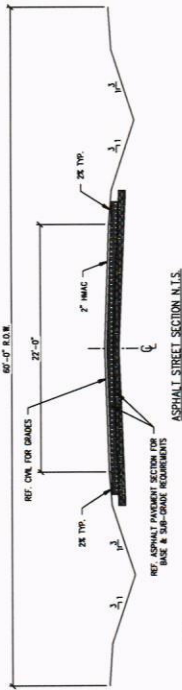
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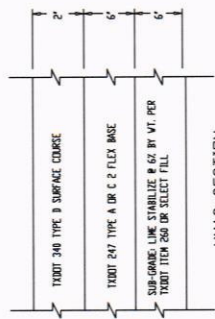
NAME	DATE
DESIGNED BY	11/11/2020
CHECKED BY	11/11/2020
DATE	11/11/2020

PROJECT # 210
SHEET # 110
SHEET NUMBER T1.0



NOTES:
 1. A SOIL INVESTIGATION FOR SUB-GRADE DESIGN SHALL BE CONDUCTED
 2. BASE COURSE WIDTHS (WHEN APPLICABLE) 24'
 3. SUB-GRADE WIDTH 28'

HOT MIX ASPHALTIC
 CONCRETE PAVEMENT



ASPHALT PAVEMENT SECTION
 SCALE NINE

PERM: 1/16/2020
 ISSUED FOR: DATE:

BECKHAM SUB-DIVISION
 SULPHUR SPRINGS, TX



PROFESSIONAL
 ENGINEERING
 SERVICES
 208 S. 24th STREET, G
 SULPHUR SPRINGS, TX
 75483
 (940) 261-1111
 www.dynamic-engineering.com



ENGINEERING SEAL	NAME	DATE
CHECKED	ELUCONCELI	1/16/20
DESIGNED	ELUCONCELI	1/16/20
APP. APPR.	ELUCONCELI	1/16/20
DATE		

PROJECT # 7179
 SHEET NUMBER C1.1

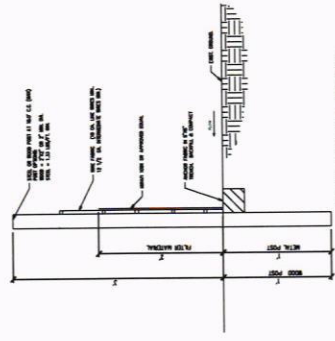
EROSION CONTROL SEQUENCE	
1.	CONSTRUCT TEMPORARY CONSTRUCTION ERI.
2.	INSTALL PERIMETER SILT FENCE AS SHOWN.
3.	COMMENCE GRUBBING AND REMOVAL OF VEGETATION IN AREAS TO RECEIVE CUT OR FILL.
4.	CONSTRUCT DETENTION POND (IF APPLICABLE) WITH PERMANENT OUTLET STRUCTURE, TO BE USED AS TEMPORARY SEDIMENT BASIN, WITH TEMPORARY OUTLET CONTROL. (SEE DETAILS).
5.	COMMENCE GRADING OPERATION FOR BUILDING PAD PREPARATION (SEE GRADING PLAN).
6.	INSTALL ALL UNDERGROUND UTILITIES.
7.	FINALIZE PAVEMENT SUB-GRADE.
8.	INSTALL ALL PROTECTION AROUND NEWLY CONSTRUCTED INLETS.
9.	INSTALL BASE MATERIAL, AS REQUIRED FOR PAVEMENT, CURB, AND GUTTER.
10.	INSTALL ALL PAVING, CURB AND GUTTER.
11.	COMPLETE PLANTING AND/OR SEEDING OF VEGETATED AREAS TO ACCOMPLISH STABILIZATION, IN ACCORDANCE WITH THE LANDSCAPING PLAN.
12.	REMOVE TEMPORARY DETENTION POND, REMOVE TEMPORARY OUTLET STRUCTURE, AND EXCAVATE ACCUMULATED SEDIMENT TO RETURN TO ORIGINAL GRADE OR TO TOP OF EXISTING CONSTRUCTION.
13.	REMOVE ALL TEMPORARY SEDIMENT CONTROL MEASURES AND CONSTRUCTION ERI.

**** NOTICE TO CONTRACTORS ****

TOPOGRAPHIC INFORMATION TAKEN FROM A TOPOGRAPHIC SURVEY PERFORMED BY EST, INC. THE CONTRACTOR SHALL NOTIFY THE ENGINEER IMMEDIATELY IN WRITING, OF ANY DISCREPANCIES OR OMISSIONS TO THE INFORMATION PROVIDED TO THE CONTRACTOR. THE CONTRACTOR SHALL NOTIFY THE OWNER AND ENGINEER OF ANY DISCREPANCIES OR OMISSIONS TO THE INFORMATION PROVIDED TO THE CONTRACTOR. THE CONTRACTOR SHALL NOTIFY THE OWNER AND ENGINEER OF ANY DISCREPANCIES OR OMISSIONS TO THE INFORMATION PROVIDED TO THE CONTRACTOR. THE CONTRACTOR SHALL NOTIFY THE OWNER AND ENGINEER OF ANY DISCREPANCIES OR OMISSIONS TO THE INFORMATION PROVIDED TO THE CONTRACTOR. THE CONTRACTOR SHALL NOTIFY THE OWNER AND ENGINEER OF ANY DISCREPANCIES OR OMISSIONS TO THE INFORMATION PROVIDED TO THE CONTRACTOR.

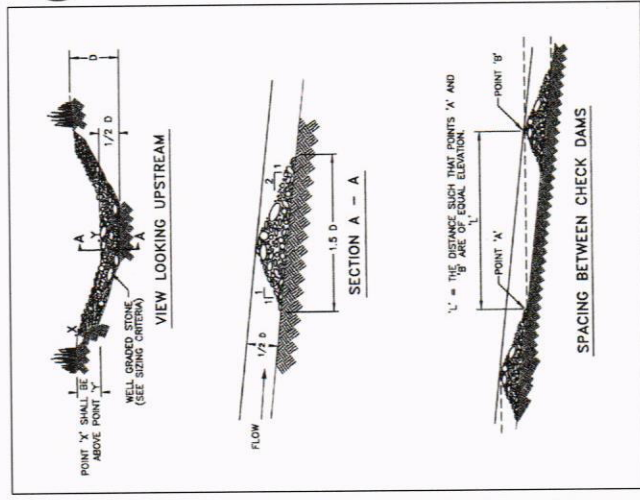
EROSION CONTROL NOTES	
1.	CONTRACTOR MUST COMPLETE A CONSTRUCTION SITE VISIT. OBTAIN SIGNED COPIES OF NOTICE FROM BOTH OWNER AND CONTRACTOR (IF APPLICABLE) AND PROVIDE TO THE CONSTRUCTION SITE, IN ACCORDANCE WITH THE TERMS POLLUTANT DISCHARGE ELIMINATION SYSTEM (TPDES) GENERAL PERMIT FOR CONSTRUCTION ACTIVITIES (21000000). THE GENERAL CONTRACTOR, AND ALL SUBCONTRACTORS AS WELL AS ALL AS THE REQUIREMENTS SET FORTH IN THE TPDES GENERAL PERMIT MUST BE FAMILIAR WITH THE CONTENTS OF THE STORM WATER POLLUTION PREVENTION PLAN (SWPPP) AS WELL AS ALL THE REQUIREMENTS SET FORTH IN THE TPDES GENERAL PERMIT AND ANY APPLICABLE LOCAL PERMIT REQUIREMENTS, AND SHALL COMPLY WITH ALL SUCH REQUIREMENTS DURING THE COURSE OF CONSTRUCTION. THE CONTRACTOR SHALL MAINTAIN RECORDS OF ALL DEVIATION FROM THIS SEDIMENT BOUND NECESSARY BY THE CONTRACTOR THAT REQUIRE THAT THE STORM WATER POLLUTION PREVENTION PLAN BE MODIFIED IN ACCORDANCE WITH THE TPDES GENERAL PERMIT GUIDELINES AND SECTION 1.01 OF THE STORM WATER POLLUTION PREVENTION PLAN. THE CONTRACTOR SHALL NOTIFY THE PLAN TO SHOW LOCATIONS OF TEMPORARY WASH DOWNS, PORTABLE TOILETS, EQUIPMENT MAINTENANCE/PREPARE AREAS, FUEL STORAGE AREAS, AND POLLUTANT CONTROL AREAS, AS SOON AS POSSIBLE.
2.	THE GENERAL CONTRACTOR SHALL PERFORM ALL REQUIRED INSPECTIONS OF STORM WATER CONTROLS AND PRACTICES AT FREQUENCIES GIVEN IN THE TPDES GENERAL PERMIT, AND SHALL COMPLETE AND SIGN APPROPRIATE INSPECTION FORMS (AS PROVIDED IN THE STORM WATER POLLUTION PREVENTION PLAN).
3.	SOIL AND GREASE ABSORBING MATERIALS SHALL BE COMPLETELY AVAILABLE ON-SITE AND SHALL BE PROMPTLY USED TO CONTAIN AND/OR CLEAN UP ALL FUEL OR OIL/GREASE SPILLS ON LOCAL.
4.	SOIL CONTROL MEASURES FOR ALL EXPOSED AREAS ON A REGULAR BASIS. SPRAYING OF PETROLEUM BASED STABILIZERS FOR THE PURPOSE IS PROHIBITED.
5.	DISTURBED AREAS OF THE SITE WHERE CONSTRUCTION ACTIVITIES HAVE CEASED FOR AT LEAST FOURTEEN DAYS SHALL BE TEMPORARILY STABILIZED WITH VEGETATION AND MULCH.
6.	DISBURBED AREAS OF THE SITE WHERE CONSTRUCTION ACTIVITIES HAVE PERMANENTLY CEASED SHALL BE PERMANENTLY SEEDED WITH FOURTEEN DAYS PER LANDSCAPING SPECIFICATIONS.
7.	ALL VEHICLES SHALL BE CLEANED AT THE CONSTRUCTION SITE EXIT POINTS ACCORDING TO NOTICES POSTED ON THE SITE. TRUCKS, IF NECESSARY, SHALL BE WASHED AT THE CONSTRUCTION SITE. VEHICLES SHALL BE WASHED BEFORE EXITING ONTO PUBLIC ROADS. SILT FROM THE WASHING OPERATION SHALL BE INTERCEPTED AND TRAPPED BEFORE WASH WATER IS ALLOWED TO BE DISCHARGED OFF-SITE.
8.	ALL MATERIALS SPILLED, DROPPED, WASHED OR TRACKED ONTO ADJACENT HIGHWAYS BY VEHICLES EXITING THE SITE SHALL BE CLEANED OR REMOVED IMMEDIATELY.
9.	ALL MATERIALS SPILLED, DROPPED, WASHED OR TRACKED ONTO ADJACENT HIGHWAYS BY VEHICLES EXITING THE SITE SHALL BE CLEANED OR REMOVED IMMEDIATELY.
10.	THE CONTRACTOR SHALL REMOVE ALL ACCUMULATED SILT IN ANY TEMPORARY OR PERMANENT DETENTION PONDS, STORM SEWER INLETS AND PIPES, AND ALONG SILT FENCES, WITHIN 48 HOURS AFTER INSPECTION OF DEVICES REVEALS THE PRESENCE OF EXCESSIVE SILTATION (AS SPECIFIED IN SECTION 5.02 OF THE STORM WATER POLLUTION PREVENTION PLAN).
11.	SILT FENCES SHALL BE PLACED AROUND ANY STOCKPILES USED ON THE SITE.
12.	THE CONTRACTOR IS ADVISED TO CONSTRUCT TEMPORARY OR PERMANENT FENCING AROUND DETENTION PONDS AND SEDIMENT BASINS AT THE EARLIEST POSSIBLE TIME TO PREVENT ACCIDENTAL ACCESS BY PERSONS OR ANIMALS.
13.	ANY ADDITIONAL EROSION CONTROL MEASURES NOT SPECIFIED IN THIS PLAN SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR, AT NO ADDITIONAL CHARGE TO THE OWNER. ALL EROSION CONTROL MEASURES SHALL BE REMOVED AND PROPERLY DISPOSED OF OFF-SITE WITHIN THIRTY DAYS AFTER COMPLETION OF ALL SURFACES.
14.	THE CONTRACTOR SHALL ASSUME LIABILITY FOR DAMAGE TO ADJACENT PROPERTIES AND/OR PUBLIC RIGHT-OF-WAY RESULTING FROM FAILURE TO FULLY IMPLEMENT AND EXECUTE ALL EROSION CONTROL PROCEDURES SHOWN AND NOTED IN THESE PLANS.
15.	WHENEVER DIRT, ROCK, OR OTHER MATERIALS ARE EXPLORED FOR USE OFF OF THE PRIMARY CONSTRUCTION SITE, CONTRACTOR SHALL ASSUME RESPONSIBILITY FOR CONSTRUCTION MANAGEMENT WITH DOCUMENTATION OF COVERAGE FOR THE BURNING OF FILL SITE UNDER A TPDES PERMIT FOR STORMWATER DISCHARGES AND OF A WRITTEN AGREEMENT WITH THE LANDOWNER. ALL MATERIALS MUST BE COVERED WITH EROSION CONTROL DEVICES (SILT FENCES) ON ALL DOWN SLOPES AND SAND SLOPE BOUNDARIES OF ANY DISTURBED AREA, PLUS PROVISIONS FOR RE-VEGETATION AFTER THE FILL MATERIALS ARE IN PLACE.
16.	ALL SLOPES ON SITE WHICH ARE 3:1 OR STEEPER SHALL BE STABILIZED BY TRACK WALKING (TRAVELING UP AND DOWN THE SLOPE) AND MULCHING WITH EROSION CONTROL BLANKET SHALL BE NORTH AMERICAN GREEN 5150 OR APPROVED EQUAL.

SWPPP MAINTENANCE NOTES	
1.	ALL MEASURES STATED ON THIS EROSION AND SEDIMENT CONTROL PLAN, AND IN THE STORM WATER POLLUTION PREVENTION PLAN SHALL BE MAINTAINED AT ALL TIMES AND SHALL BE SUBJECT TO INSPECTION AND VERIFICATION BY THE ENGINEER AND/OR A QUALIFIED PERSON ON A SCHEDULE WHICH COMPLES WITH THE GENERAL PERMIT REQUIREMENTS AND CLEANED AND REPAIRED WITHIN 48 HOURS OF THE INSPECTION IN ACCORDANCE WITH THE FOLLOWING:
2.	INLET PROTECTION DEVICES AND BARRIERS SHALL BE REPAIRED OR REPLACED IF THE SHOW SIGNS OF UNDERMINING, OR DETEIORATION, AND REPAIRED AS NECESSARY TO PREVENT FURTHER DAMAGE TO THE STRUCTURE. SEDIMENT SHALL BE REMOVED FROM THE SILT FENCES WHEN IT REACHES ONE-HALF THE HEIGHT OF THE SILT FENCE.
3.	THE TEMPORARY PAVING AND STORAGE AREA (IF PRESENT) SHALL BE KEPT IN GOOD CONDITION (SUITABLE FOR PARKING AND STORAGE). THIS MAY REQUIRE PERIODIC TOP DRESSING OF THE PAVING AS CONDITIONS DEMAND.
4.	OUTLET STRUCTURES IN THE SEDIMENTATION BASINS OR SEDIMENT TRAPS (IF PRESENT) SHALL BE MAINTAINED IN OPERATIONAL CONDITION AT ALL TIMES. SEDIMENT SHALL BE REMOVED FROM SEDIMENT BASINS OR TRAPS WHEN THE DESIGN CAPACITY HAS BEEN REDUCED BY SOLE PLAN.
5.	MAINTENANCE PROCEDURES FOR THE EROSION AND SEDIMENTATION CONTROL SYSTEMS ARE SPECIFIED IN THE STORM WATER POLLUTION PLAN.

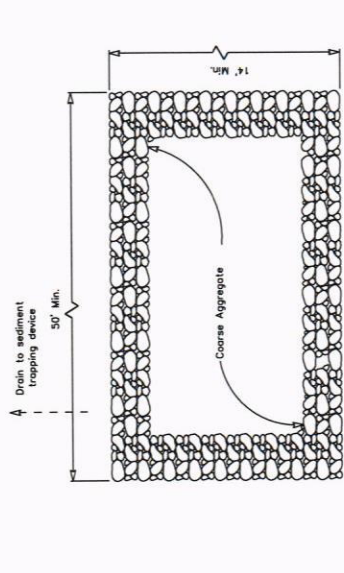


SILT FENCE DETAIL
N.T.S.

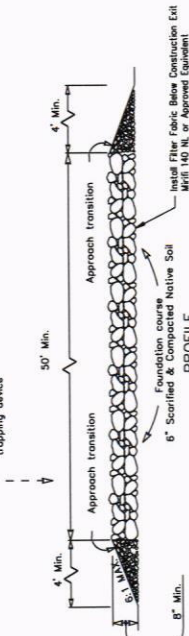
NOTE: STRAW BALES MAY BE USED IN LIEU OF FABRIC AROUND CATCH BASIN. STRAW BALES SHOULD BE SECURELY STAKED IN PLACE.



ROCK BERM DETAIL
N.T.S.

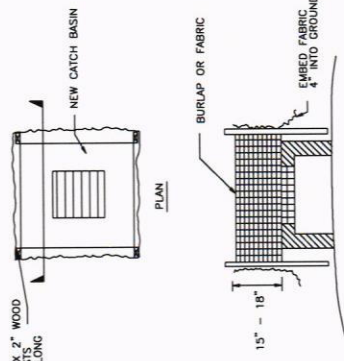


CONSTRUCTION EXIT
N.T.S.

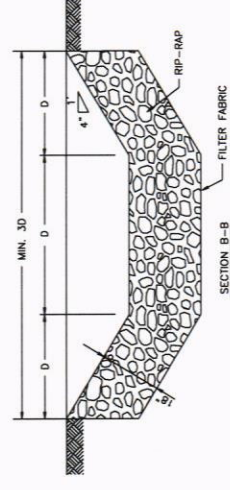
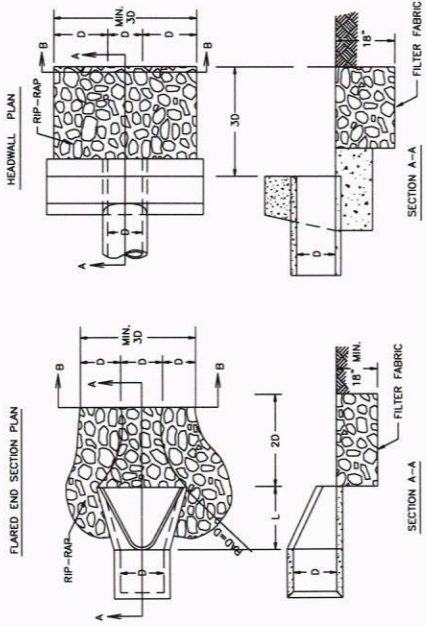


GENERAL NOTES

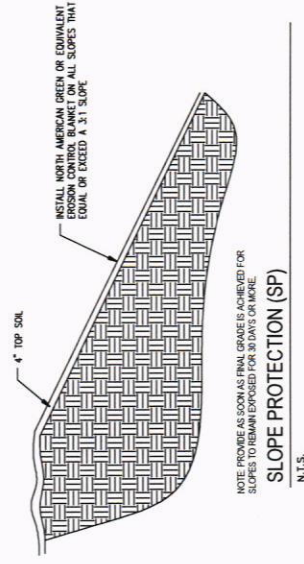
1. The length of the 1' construction exit shall be as indicated on the plans, but not less than 50'.
2. The coarse aggregate should be open graded with a size of 4" to 8".
3. The approach transitions should be no steeper than 6:1.
4. The construction exit shall be graded to allow drainage to a sediment trapping device.



CATCH BASIN PROTECTION
N.T.S.



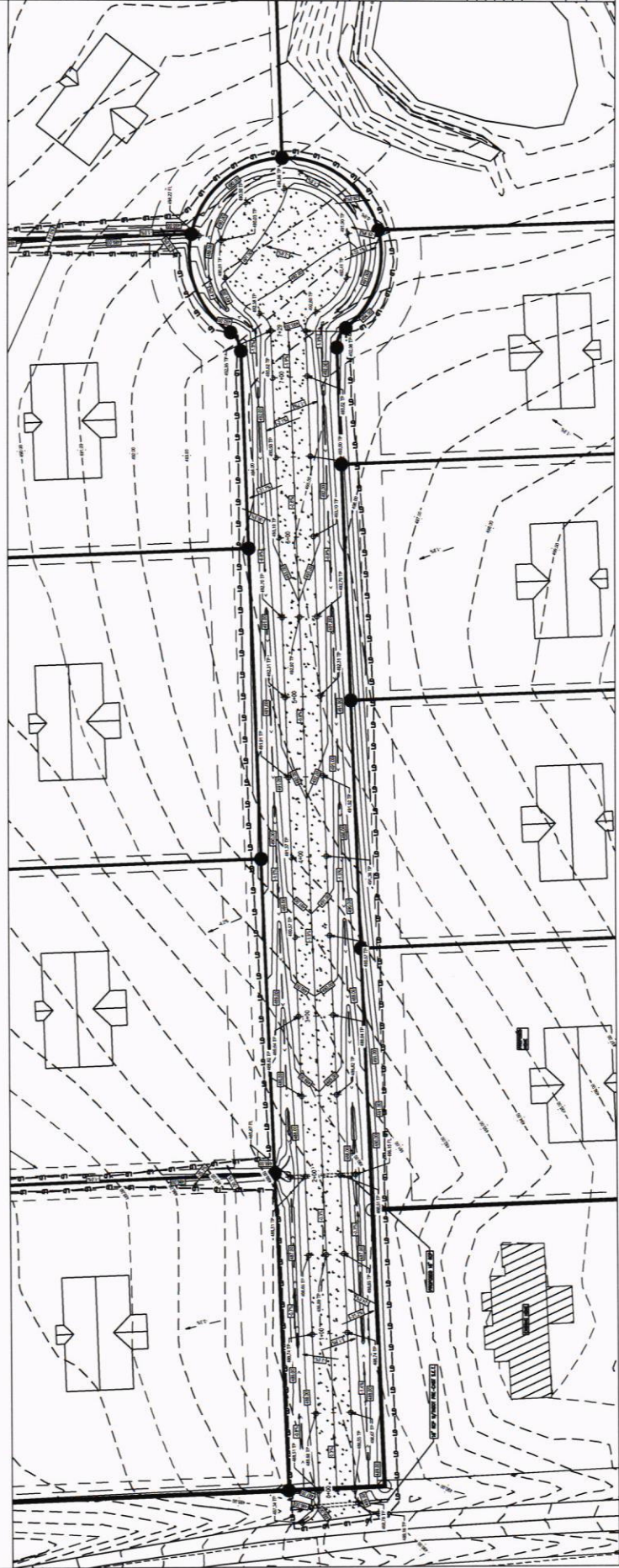
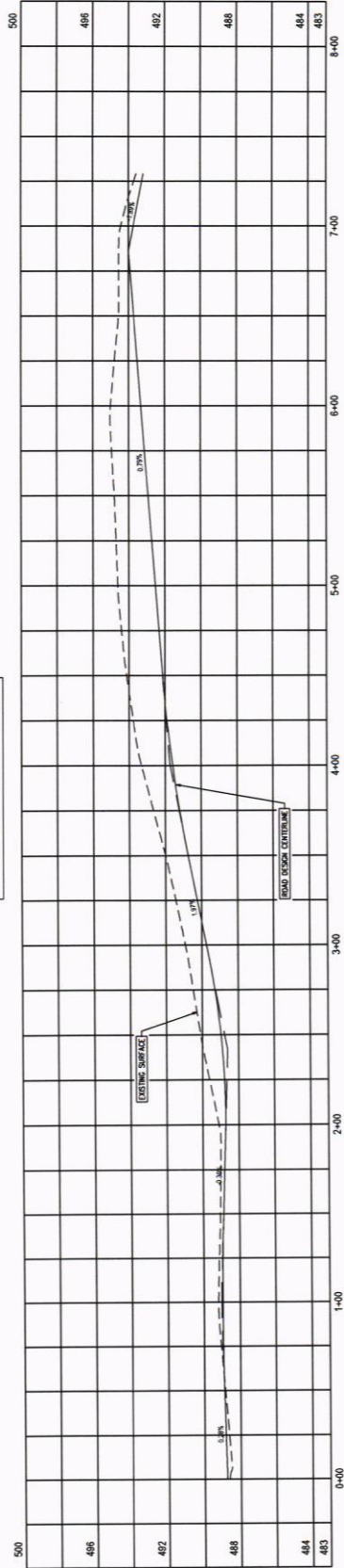
DISCHARGE AREA EROSION CONTROL DETAIL
N.T.S.



SLOPE PROTECTION (SP)
N.T.S.

NOTE: PROTECT AS MUCH AS PRACTICABLE FOR SLOPES TO REMAIN EXPOSED FOR 30 DAYS OR MORE.

PROPOSED ROAD PROFILE



BECKHAM SUB-DIVISION
SULPHUR SPRINGS, TX

PERMIT: 16A0200
ISSUED FOR: DATE:



PROFESSIONAL
ENGINEERS
SERVICES
200 N. HILLCREST, SUITE C
SULPHUR SPRINGS, TX
75483
PHONE: 817.338.6000
FAX: 817.338.6001
WWW.DYNAMICCONSULTANTS.COM



REVISION	DATE

FOR THE PROJECT MANAGER:
NAME: [Signature]
TITLE: PROJECT MANAGER
DATE: [Signature]

FOR THE ENGINEER:
NAME: [Signature]
TITLE: PROFESSIONAL ENGINEER
DATE: [Signature]

PROPOSED ROAD PLANS & PROFILE
SHEET NUMBER: C3.1

Drainage Divide	Area (Acre)	Time of Conc. t (min)	Composite Runoff	Average Slope S (%)	Overland Flow	1 yr	2 yr	5 yr	10 yr	25 yr	50 yr	100 yr
					Flow	Qa(CFS)	Qa(CFS)	Qa(CFS)	Qa(CFS)	Qa(CFS)	Qa(CFS)	Qa(CFS)
Pre-A1	11.17	30	0.26	3.5	250	6.88	8.04	10.05	11.36	14.50	17.60	20.26
Pre-A2	10.32	30	0.26	3.5	250	6.36	7.43	9.28	10.49	13.40	16.26	18.75
Pre-A3	5.00	30	0.26	3.5	250	3.08	3.60	4.50	5.09	6.50	7.88	9.09
TOTAL:						16.32	19.08	23.83	26.93	34.40	41.74	48.13

Drainage Divide	Area (Acre)	Time of Conc. t (min)	Composite Runoff	Average Slope S (%)	Overland Flow	1 yr	2 yr	5 yr	10 yr	25 yr	50 yr	100 yr
					Flow	Qa(CFS)	Qa(CFS)	Qa(CFS)	Qa(CFS)	Qa(CFS)	Qa(CFS)	Qa(CFS)
Post-A1	11.17	25	0.30	4	100	8.88	10.35	12.80	14.48	18.47	22.36	25.76
Post-A2	10.32	30	0.26	4	100	6.36	7.43	9.28	10.49	13.40	16.26	18.75
Post-A3	5.00	25	0.30	4	100	3.98	4.64	5.73	6.48	8.27	10.01	11.53
TOTAL:						19.21	22.42	27.81	31.45	40.13	48.63	56.04

- GENERAL NOTES:
1. DRAINAGE FREQUENCY - 100 YEARS
2. RATIONAL METHOD AS DEFINED IN THE SWM DESIGN MANUAL FOR SORT DEVELOPMENT WAS USED TO DETERMINE THE PRE-DEVELOPMENT & POST DEVELOPMENT FLOW RATES.

Return Period (Yr)	Rainfall Intensity @ tc (in/hr)	Frequency Factor C
1	2.37	1
2	2.77	1
5	3.46	1
10	3.91	1
25	4.54	1.1
50	5.05	1.2
100	5.59	1.25

Return Period (Yr)	Rainfall Intensity @ tc (in/hr)	Frequency Factor C
1	2.65	1
2	3.09	1
5	3.82	1
10	4.32	1
25	5.01	1.1
50	5.56	1.2
100	6.15	1.25

POINT: _____

SUBSID FOR: _____

1/16/2010 DATE: _____

BECKHAM SUB-DIVISION
SULPHUR SPRINGS, TX

DYNAMIC
Engineering
Consultants

PROFESSIONAL ENGINEERING
OFFICE
200 S. MCGRAW-HILL
SULPHUR SPRINGS, TX

STATE OF TEXAS
PROFESSIONAL ENGINEER SEAL
19883

PRE & POST DEV. CALCULATIONS
PROJECT # _____
SHEET NUMBER
C3.4